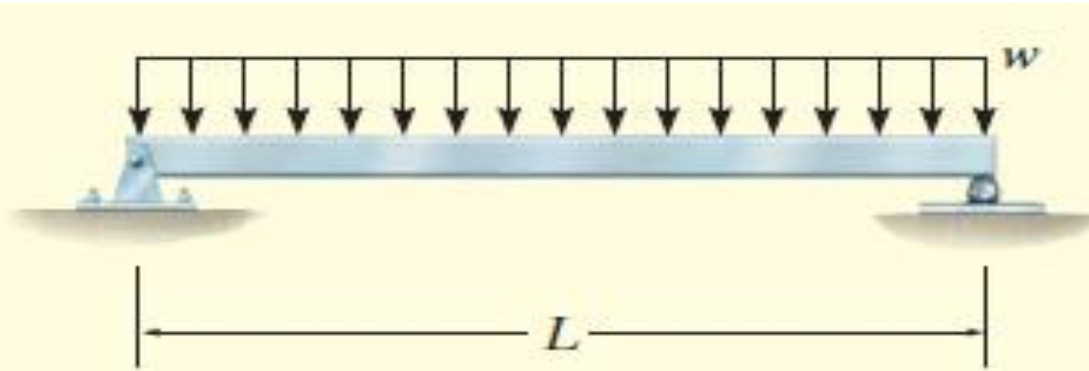
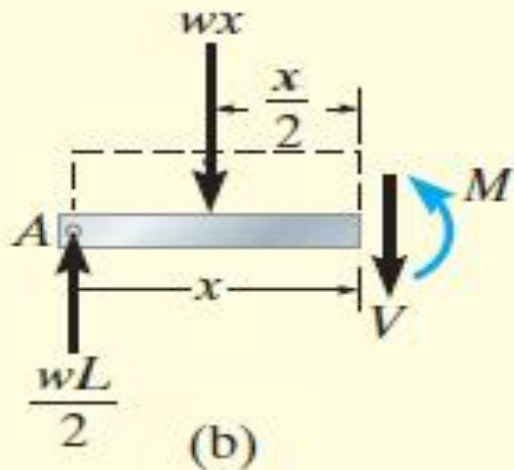


Example 1 Draw the shear and moment diagrams for the beam shown in Fig. 6–4a.



(a)



(b)

$$+\uparrow \Sigma F_y = 0; \quad \frac{wL}{2} - wx - V = 0$$

$$V = w\left(\frac{L}{2} - x\right) \quad (1)$$

$$\downarrow + \Sigma M = 0; \quad -\left(\frac{wL}{2}\right)x + (wx)\left(\frac{x}{2}\right) + M = 0$$

$$M = \frac{w}{2}(Lx - x^2) \quad (2)$$

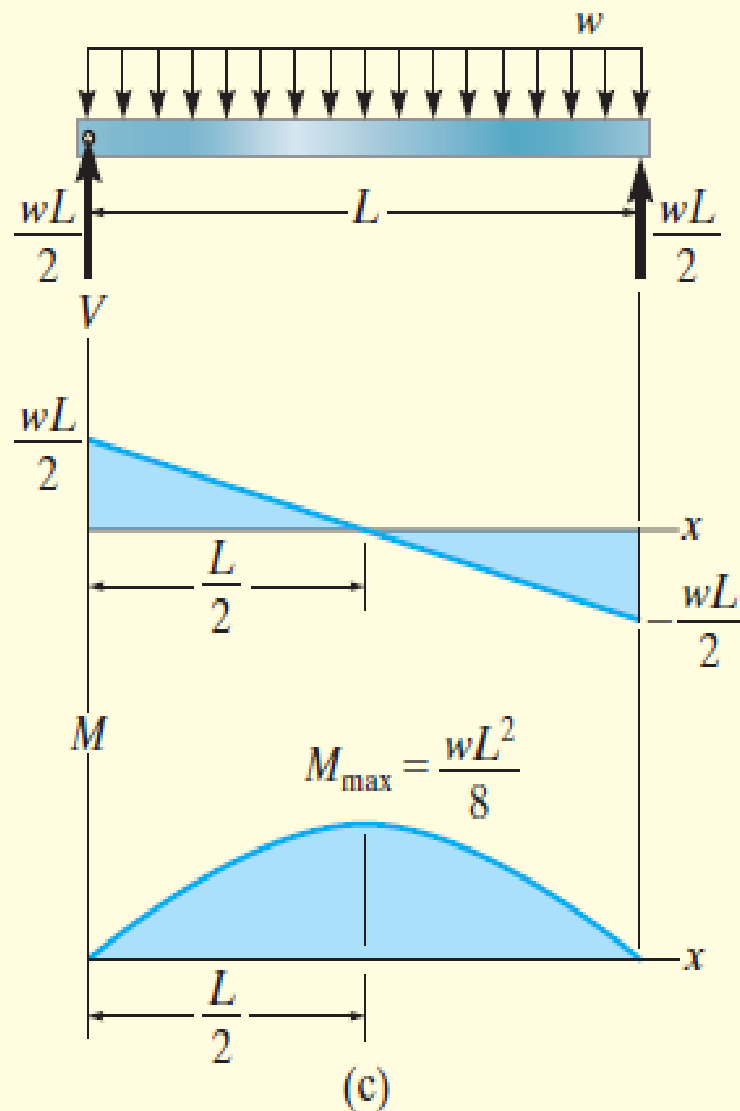


Fig. 6-4

Shear and Moment Diagrams. The shear and moment diagrams shown in Fig. 6-4c are obtained by plotting Eqs. 1 and 2. The point of *zero shear* can be found from Eq. 1:

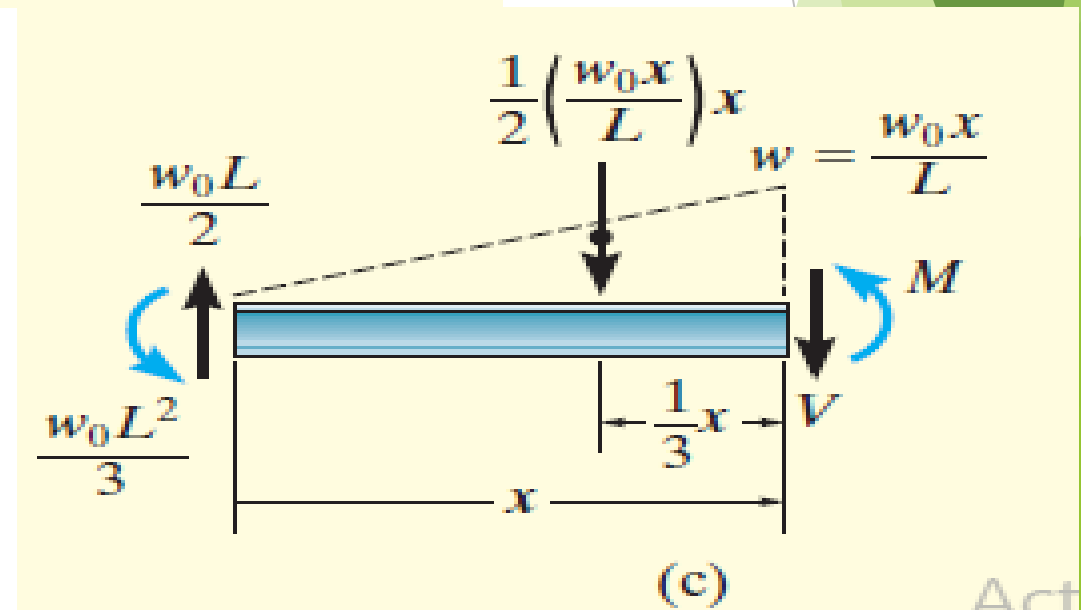
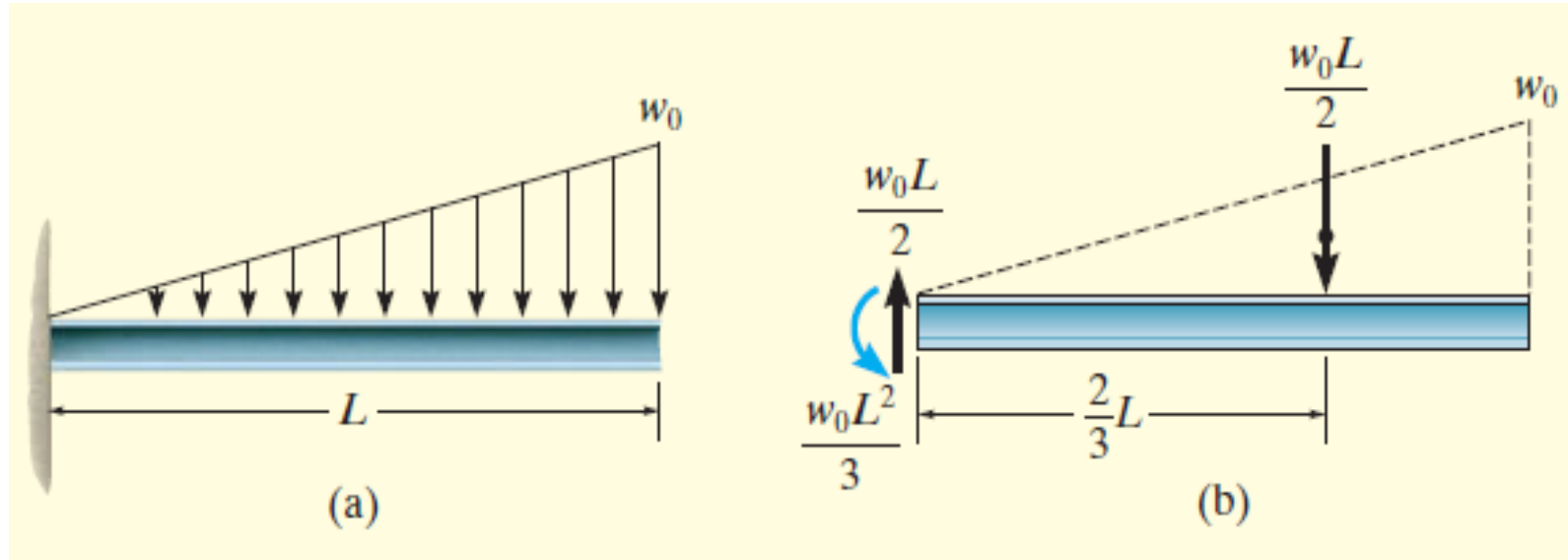
$$V = w\left(\frac{L}{2} - x\right) = 0$$

$$x = \frac{L}{2}$$

NOTE: From the moment diagram, this value of x represents the point on the beam where the *maximum moment* occurs, since by Eq. 6-2 (see Sec. 6.2) the *slope* $V = dM/dx = 0$. From Eq. 2, we have

$$\begin{aligned} M_{\max} &= \frac{w}{2} \left[L\left(\frac{L}{2}\right) - \left(\frac{L}{2}\right)^2 \right] \\ &= \frac{wL^2}{8} \end{aligned}$$

Example 2: Draw the shear and moment diagrams for the beam shown in Fig. 6–5a.



$$+\uparrow \Sigma F_y = 0; \quad \frac{w_0 L}{2} - \frac{1}{2} \left(\frac{w_0 x}{L} \right) x - V = 0$$

$$V = \frac{w_0}{2L} (L^2 - x^2) \quad (1)$$

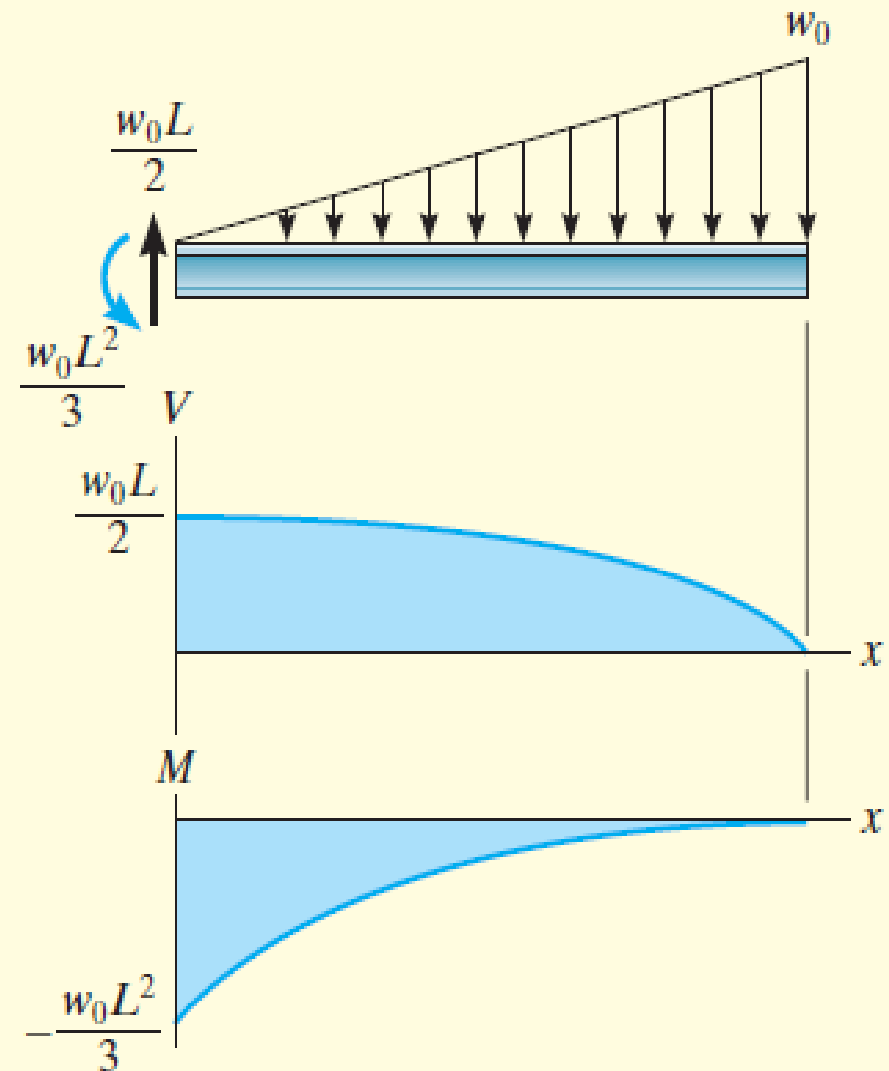
$$\downarrow + \Sigma M = 0; \quad \frac{w_0 L^2}{3} - \frac{w_0 L}{2} (x) + \frac{1}{2} \left(\frac{w_0 x}{L} \right) x \left(\frac{1}{3} x \right) + M = 0$$

$$M = \frac{w_0}{6L} (-2L^3 + 3L^2 x - x^3) \quad (2)$$

These results can be checked by applying Eqs. 6-1 and 6-2 of Sec. 6.2, that is,

$$w = \frac{dV}{dx} = \frac{w_0}{2L} (0 - 2x) = -\frac{w_0 x}{L} \quad \text{OK}$$

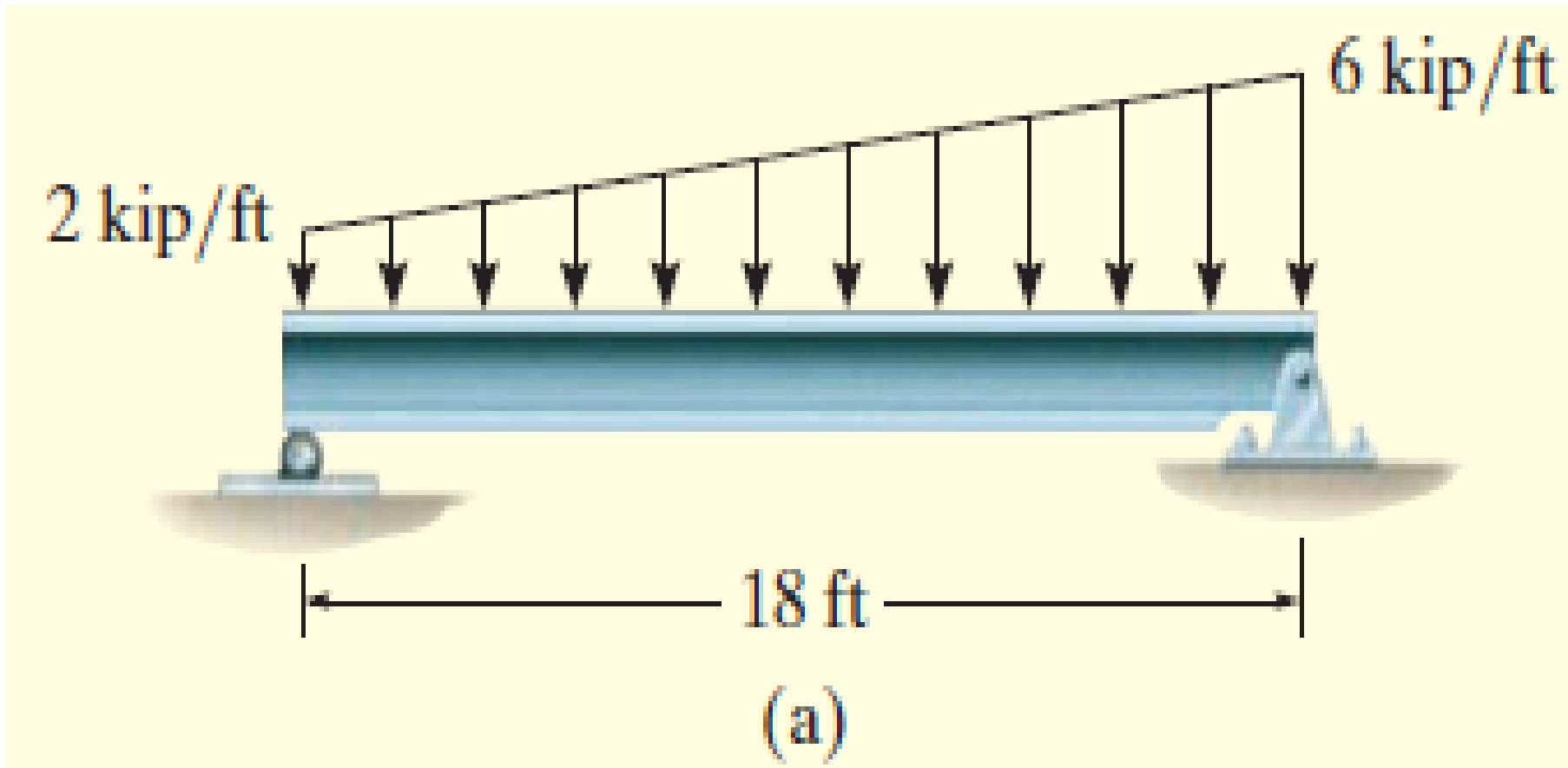
$$V = \frac{dM}{dx} = \frac{w_0}{6L} (0 + 3L^2 - 3x^2) = \frac{w_0}{2L} (L^2 - x^2) \quad \text{OK}$$

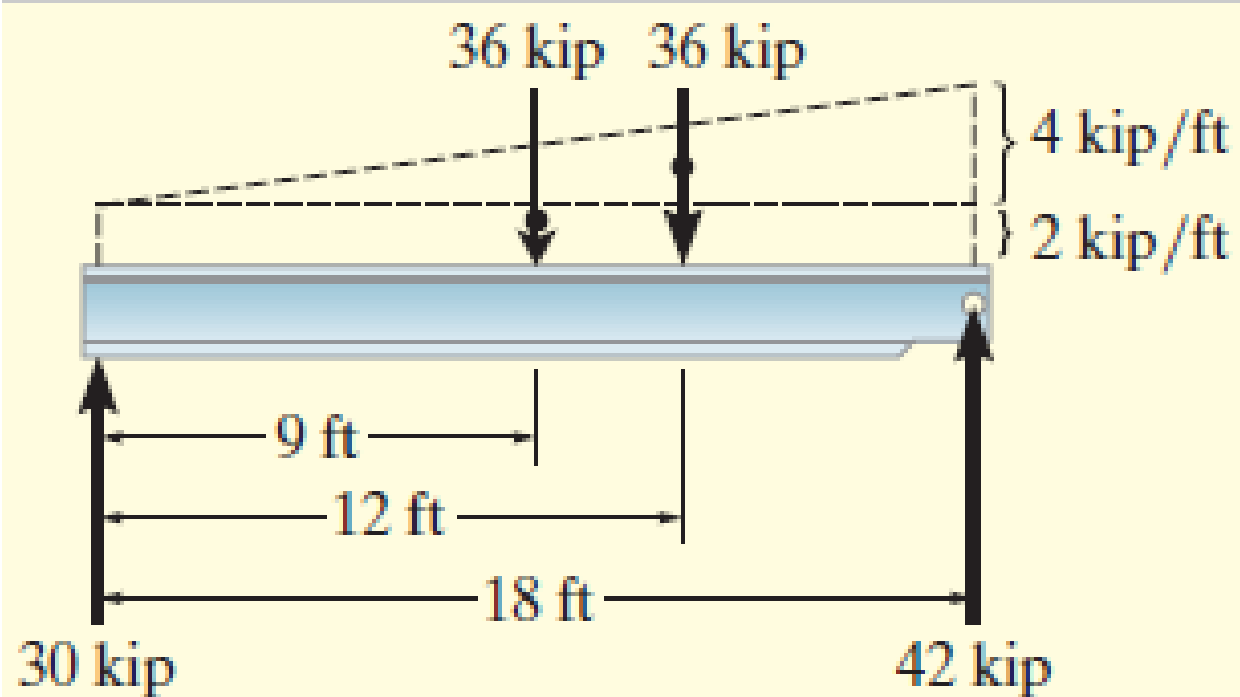


(d)

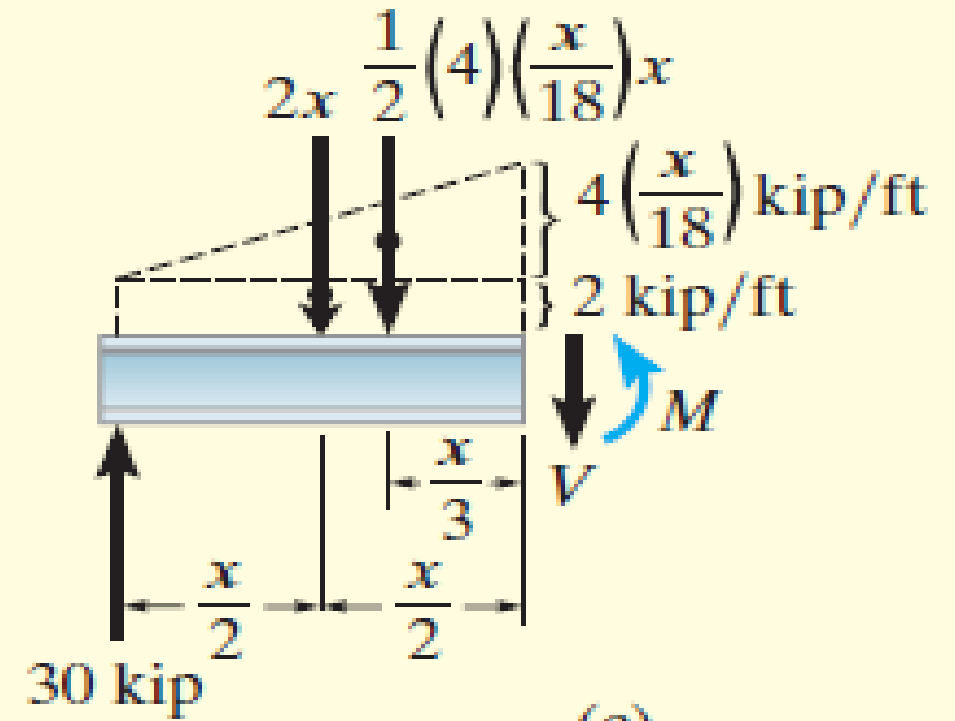
EXAMPLE 3

Draw the shear and moment diagrams for the beam shown in Figure.





(b)



(c)

$$+\uparrow \Sigma F_y = 0; 30 \text{ kip} - (2 \text{ kip/ft})x - \frac{1}{2}(4 \text{ kip/ft})\left(\frac{x}{18 \text{ ft}}\right)x - V = 0$$

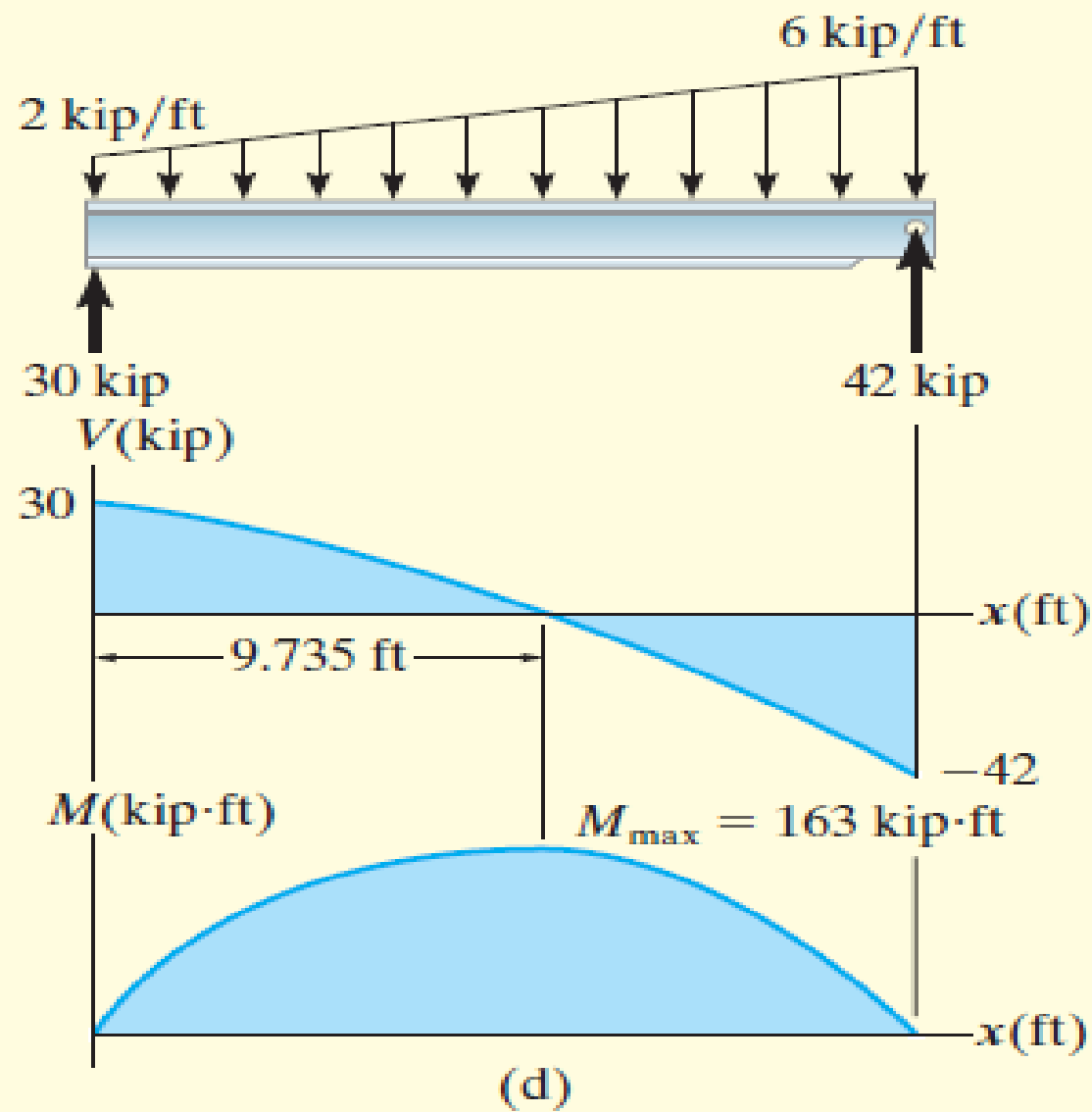
$$V = \left(30 - 2x - \frac{x^2}{9}\right) \text{ kip} \quad (1)$$

$$\downarrow + \Sigma M = 0;$$

$$-30 \text{ kip}(x) + (2 \text{ kip/ft})x\left(\frac{x}{2}\right) + \frac{1}{2}(4 \text{ kip/ft})\left(\frac{x}{18 \text{ ft}}\right)x\left(\frac{x}{3}\right) + M = 0$$

$$M = \left(30x - x^2 - \frac{x^3}{27}\right) \text{ kip} \cdot \text{ft} \quad (2)$$

Equation 2 may be checked by noting that $dM/dx = V$, that is, Eq. 1. Also, $w = dV/dx = -2 - \frac{2}{9}x$. This equation checks, since when $x = 0$, $w = -2 \text{ kip/ft}$, and when $x = 18 \text{ ft}$, $w = -6 \text{ kip/ft}$, Fig. 6-6a.



Shear and Moment Diagrams. Equations 1 and 2 are plotted in Fig. 6-6d. Since the point of maximum moment occurs when $dM/dx = V = 0$ (Eq. 6-2), then, from Eq. 1,

$$V = 0 = 30 - 2x - \frac{x^2}{9}$$

Choosing the positive root,

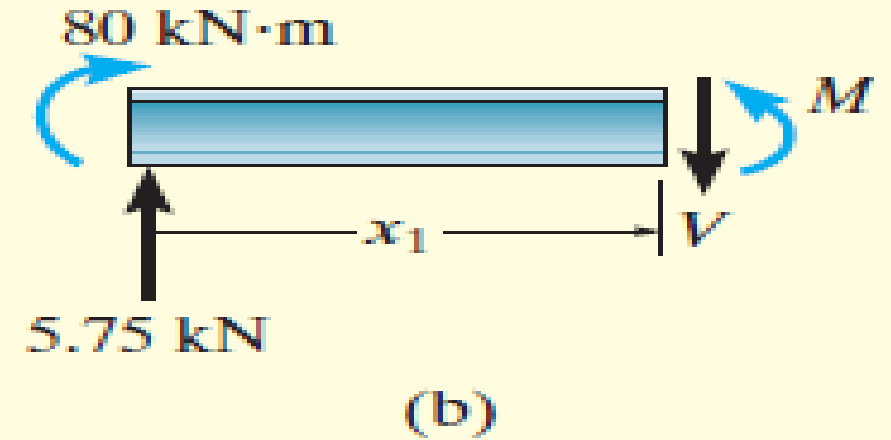
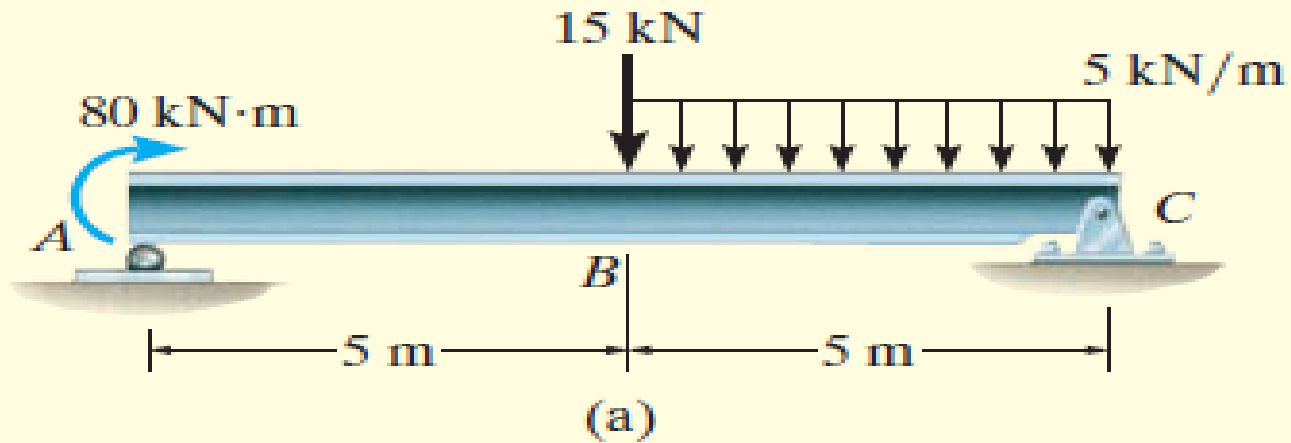
$$x = 9.735 \text{ ft}$$

Thus, from Eq. 2,

$$\begin{aligned} M_{\max} &= 30(9.735) - (9.735)^2 - \frac{(9.735)^3}{27} \\ &= 163 \text{ kip}\cdot\text{ft} \end{aligned}$$

EXAMPLE 4

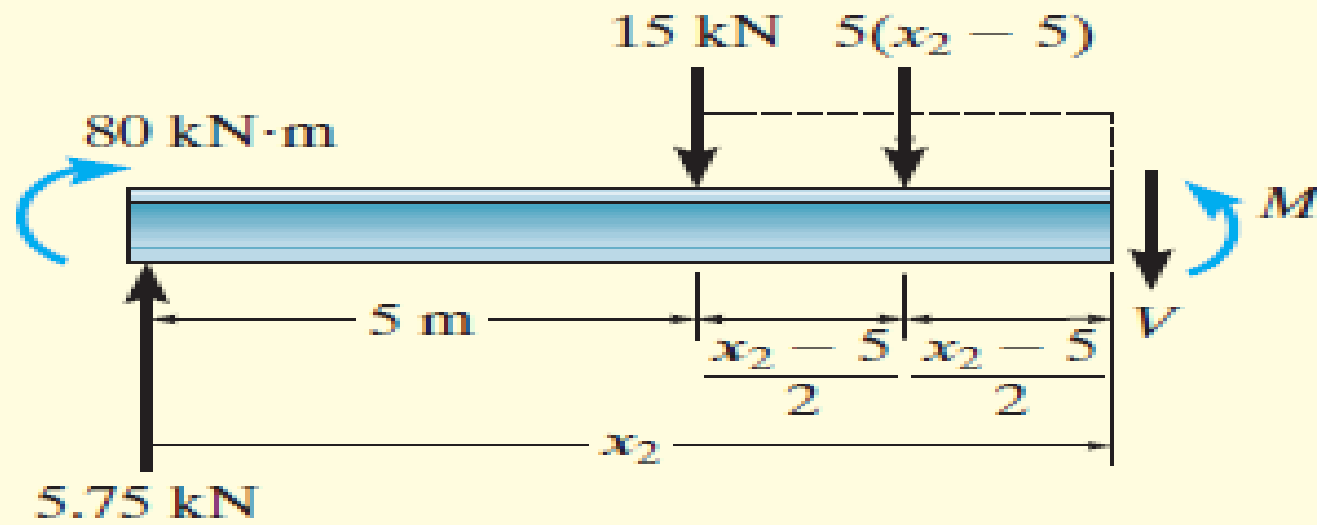
Draw the shear and moment diagrams for the beam shown in Figure.



$0 \leq x_1 < 5 \text{ m}$, Fig. 6-7b:

$$+\uparrow \Sigma F_y = 0; \quad 5.75 \text{ kN} - V = 0$$
$$V = 5.75 \text{ kN}$$

$$+\circlearrowleft \Sigma M = 0; \quad -80 \text{ kN} \cdot \text{m} - 5.75 \text{ kN } x_1 + M = 0$$
$$M = (5.75x_1 + 80) \text{ kN} \cdot \text{m}$$



(c)

$5 \text{ m} < x_2 \leq 10 \text{ m}$, Fig. 6–7c:

$$+\uparrow \Sigma F_y = 0; \quad 5.75 \text{ kN} - 15 \text{ kN} - 5 \text{ kN/m}(x_2 - 5 \text{ m}) - V = 0$$

$$V = (15.75 - 5x_2) \text{ kN} \quad (3)$$

$$\zeta + \Sigma M = 0; \quad -80 \text{ kN} \cdot \text{m} - 5.75 \text{ kN } x_2 + 15 \text{ kN}(x_2 - 5 \text{ m})$$

$$+ 5 \text{ kN/m}(x_2 - 5 \text{ m})\left(\frac{x_2 - 5 \text{ m}}{2}\right) + M = 0$$

$$M = (-2.5x_2^2 + 15.75x_2 + 92.5) \text{ kN} \cdot \text{m} \quad (4)$$

These results can be checked in part by noting that $w = dV/dx$ and $V = dM/dx$. Also, when $x_1 = 0$, Eqs. 1 and 2 give $V = 5.75 \text{ kN}$ and $M = 80 \text{ kN}\cdot\text{m}$; when $x_2 = 10 \text{ m}$, Eqs. 3 and 4 give $V = -34.25 \text{ kN}$ and $M = 0$. These values check with the support reactions shown on the free-body diagram, Fig. 6-7d.

